

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

CABARRUS COUNTY

LOCATION: REPLACE BRIDGE NO. 120105 OVER BIG BEAR CREEK ON SR 2450 (GOLD HILL RD.)

TYPE OF WORK: GRADING, PAVING, DRAINAGE AND STRUCTURE

	BEGIN CONSTRUCTION I7BP.IO.R.IIO -L- STA.I3+69.00	SI SI
	SR 2450 GOLD HILL RD	S END BRIDGE -L- STA. 17+61.3
	BEGIN BRIDGE -L- STA.16+88.70 st st	<u>END PROJECT I7BP.10.R.110</u> -L- STA.19+35.00
<u>R.//0_</u> / 5.00	BIG BEAR CREEK BRANCH	<u>END CONSTRU</u> -L- STA.2I+2

PROJECT LENGTH		
LENGTH OF ROADWAY STATE PROJECT 17BP.10.R.110	=	.088 MILES
LENGTH OF STRUCTURE STATE PROJECT 17BP.10.R.110	=	.014 MILES
TOTAL LENGTH OF STATE PROJECT 17BP.10.R.110	=	.102 MILES



NCDOT CONTACT:

YANWEI MA, P.E. DIVISION 10 BRIDGE PROGRAM MANAGER



PLAN						
FOOTINGS	NOT	SHOWN	IN	PLAN	VIEW	

Ì		I	

	PROJE	ECT NC)17	BP.IO.R	. IIO		
	CABARRUS COUNTY						
	STAT	ION:	17+25.	00 -L·			
	SHEET 1 O	F 5	REPLACES	BRIDGE N	120105		
Robert Decola	DEP	ARTMENT	e of north car OF TRA raleigh	NSPORTA	TION		
TH CARO	G	ENER	AL DF	NAWII	NG		
SEAL 36321	FOR E BI BET	BRIDGE G BEAF WEEN S	ON SF R CREE SR 266	R 2450 K BRAN 3 & NO	OVER NCH C 49		
1/10/2025							
KCI Associates of N.C., P.A. 4800 Falls of Neuse Road, Suite 200	NO. BY:	REVIS DATE:	NO. BY:	DATE:	SHEET NO. S-1		
Raleigh, NC 27609 Phone (919) 783-9214 NC Firm License No: C-0764	1		3 4		total sheets 17		



END BENT 1

FOUNDATION NOTES

THE SPREAD FOOTING AT END BENTS NOS.1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 20.0 KSF.CHECK FIELD CONDITIONS FOR THE REQUIRED RESISTANCE OF 45 KSF JUST BEFORE PLACING CONCRETE.

FOR BLASTING ADJACENT TO HIGHWAY STRUCTURES, SEE ARTICLE 410-9 OF THE STANDARD SPECIFICATIONS.

KEY IN SPREAD FOOTINGS AT END BENTS NOS.1 AND 2 AT LEAST 12"INTO WEATHERED ROCK WITH MINIMUM THINKNESS AS SHOWN ON THE PLANS.

ESTIMATE 5 CUBIC YARDS OF ROCK EXCAVATION FOR END BENT NO.1 FOUNDATION AND 14 CUBIC YARDS OF ROCK EXCAVATION FOR END BENT NO.2 FOUNDATION.

DESIGN ENGINE <u>R.F. DeCola</u>	ER OF RECORD:	DATE :	1/10/2025
DRAWN BY :	C.M. MURPHY	DATE :	10/22/24
CHECKED BY :_	R.F. DeCola	DATE :	10/24/24

<u>1</u>

END BENT 2

FOUNDATION LAYOUT

	PROJE	ICT NO	. 17	BP.IO.R	
		CABAR	RUS	CC	UNTY
	STATI	0N:	17+25.	00 -L	
	SHEET 2	OF 5			
Robert Decola C91B1BEB951B4FE BBBBBBBBBBBBBBBBBBBBBBBBBBBBBBBBBBB	DEP	STAT ARTMENT	e of north car OF TRA raleigh	NSPORTA	TION
SEAL 36321	FOl	JNDA	TION	LAY	OUT
1/10/2025 F. Ot Stars		REVIS	IONS		SHEET NO.
KCI Associates of N.C., P.A. 4800 Falls of Neuse Road, Suite 200	NO. BY:	DATE:	NO. BY:	DATE:	S- 2
D KCI http://www.kei.com Raleigh , NC 27609 Phone (919) 783-9214 NC Firm License No: C-0764	1 2		3 4		total sheets 17

SUMMARY OF SPREAD FOOTING INFORMATION

(BLANK ENTRIES INDICATE ITEM IS NOT APPLICABLE TO STRUCTURE)							
END BENT / BENT NO FOOTING(S) #(-#) (E.G., "BENT 1, FOOTING 1-2")	FACTORED BEARING RESISTANCE KSF	FOOTING DIMENSIONS (LENGTH × WIDTH) FT × FT	REQUIRED BEARING RESISTANCE KSF	SCOUR CRITICAL ELEVATION FT	MINIMUM BOTTOM OF FOOTING (FOOTING NO HIGHER THAN) ELEVATION FT		
END BENT 1, FOOTING	20		45		603		
END BENT 2, FOOTING	20		45		603		

NOTES: 1. THE SPREAD FOOTING FOUNDATION TABLES ARE BASED ON THE BRIDGE SUPERSTRUCTURE DESIGN AND FOUNDATION RECCOMENDATIONS SEALED BY A NORTH CAROLINA PROFESSIONAL ENGINEER (SHIPING YANG, #031 ON 09-25-2024

DESIGN ENG <u>R.F. DeCola</u>	INEER OF RECORD:	DATE :	1/10/2025
DRAWN BY :	C.M. MURPHY	DATE :	<u>10/22/24</u>
CHECKED BY		DATE :	10/24/24

DOCUM	IENT	NOT	CONS	IDEF	RED	FI
UNLESS	ALL	SIGN	ATUR	ES C	OMF	Ľ

	CABARRUS COUNTY
	STATION: 17+25.00 -L-
31361)	SHEET 3 OF 5
Robert Decola C91B1BEB951B4FF SSBBBBBBBBBBBBBBBBBBBBBBBBBBBBBBBBB	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
SEAL	MICROPILE AND SPREAD FOOTING
1/10/2025 F. F. J.	FOUNDATION TABLES
	REVISIONS SHEET NO.
NAL FTED KCI Associates of N.C., P.A. 4800 Falls of Neuse Road, Suite 200 Raleigh, NC 27609 Phone (919) 783-9214 NO Falls of Neuse Road, Suite 200 Phone (919) 783-9214	NO. BY: DATE: NO. BY: DATE: S - 3 1 3 TOTAL SHEETS 17
http://www.kci.com NC Firm License No: C-0764	

PROJECT NO. <u>17BP.10.R.110</u>



NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD. THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 17+25.00 -L-.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 20 FT TO THE LEFT AND 20 FT TO THE RIGHT SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF ONE 41'-7" STEEL BEAM WITH 20'-9" CLEAR ROADWAY TIMBER DECK ON CONCRETE ABUTMENTS AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

	- IOIAL	BILL	OF MA	IERIAL					
ON ION ENT .2 AT	UNCLASSIFIED STRUCTURE EXCAVATION @ STA 17+25.00 -L-	CLASS A CONCRETE	BRIDGE APPROACH SLABS STA. 17+25.00 -L-	REINFORCING STEEL	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0" X PRESTR CONCR CORED
М	LUMP SUM	C.Y.	LUMP SUM	LBS.	LIN.FT.	TON	SQ.YDS.	LUMP SUM	LIN.
			LUMP SUM		140.29			LUMP SUM	700.
		46.7		2254		115	130		
		40.1		2187		85	95		
М	LUMP SUM	86.8	LUMP SUM	4441	140.29	200	225	LUMP SUM	700.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES. SEE SPECIAL PROVISIONS.

		PROJECT NO. <u>178P.10.R.110</u>
-		CABARRUS COUNTY
2'-0" RESSED		STATION: <u>17+25.00</u> -L-
RETE SLABS		SHEET 4 OF 5
.FT.	DocuSigned by: Rolar vt. De col a	
.00		DEPARIMENT OF TRANSPORTATION RALEIGH
	PIFESSION K	GENERAL DRAWING
.00	SEAL 36321	FOR BRIDGE ON SR 2450 OVER BIG BEAR CREEK BRANCH
	1/10/2025	BEIWEEN SK 2663 & NC 49
	KCI Associates of N.C., P.A.	REVISIONS SHEET NO.
NAL Eted	4800 Falls of Neuse Road, Suite 200 Raleigh, NC 27609 Phone (919) 783-9214 NC Firm License No: C-0764	NO. BY: DATE: NO. BY: DATE: STOTAL 1 3 4 17 17
	<u>inga/www.ku.com</u>	

	LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS																							
	STRENGTH T LIMIT STATE SERVICE TIT LIMIT STATE																							
												ENGIA 1	. LIMI T						SERV				IAIE	4
									M	UMEN				5						M	DMENT			
		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS 9 LL)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	LIVE-LOAD FACTORS 9 LL)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	COMMENT NUMBE
		HL-93 (INVENTORY)	N/A		1.060		1.75	0.248	1.14	70′	EL	34.423	0.655	1.06	70′	EL	6.885	0.80	0.248	1.11	70′	EL	34.423	
DESI	GN	HL-93 (OPERATING)	N/A	•	1.374		1.35	0.248	1.48	70′	EL	34.423	0.655	1.37	70′	EL	6.885	N/A						<u> </u>
LUA	D	HS-20 (INVENTORY)	36.000		1.320	47.508	1.75	0.248	1.48	70′	EL	34.423	0.655	1.32	70′	EL	6.885	0.80	0.248	1.44	70'	EL	34.423	<u> </u>
		HS-20 (OPERATING)	36.000	•	1.711	61.585	1.35	0.248	1.91	70′	EL	34.423	0.655	1.71	70′	EL	6.885	N/A						Ļ
	CLE	SNSH	13.500	•	3.204	43.258	1.4	0.248	4.12	70′	EL	34.423	0.655	3.90	70′	EL	6.885	0.80	0.248	3.20	70′	EL	34.423	
		SNGARBS2	20.000	•	2.403	48.063	1.4	0.248	3.09	70′	EL	34.423	0.655	2.78	70′	EL	6.885	0.80	0.248	2.40	70′	EL	34.423	
	DIH:	SNAGRIS2	22.000	•	2.282	50.210	1.4	0.248	2.94	70′	EL	34.423	0.655	2.58	70′	EL	6.885	0.80	0.248	2.28	70'	EL	34.423	
	SV)	SNCOTTS3	27.250	•	1.595	43.463	1.4	0.248	2.05	70′	EL	34.423	0.655	1.95	70′	EL	6.885	0.80	0.248	1.59	70′	EL	34.423	
	CLE C	SNAGGRS4	34.925	•	1.339	46.755	1.4	0.248	1.72	70′	EL	34.423	0.655	1.62	70′	EL	6.885	0.80	0.248	1.34	70′	EL	34.423	
	NU U	SNS5A	35.550	•	1.309	46.526	1.4	0.248	1.68	70′	EL	34.423	0.655	1.65	70′	EL	6.885	0.80	0.248	1.31	70'	EL	34.423	
	0	SNS6A	39.950	•	1.203	48.069	1.4	0.248	1.55	70'	EL	34.423	0.655	1.50	70'	EL	6.885	0.80	0.248	1.20	70'	EL	34.423	
LEGAL LOAD		SNS7B	42.000	•	1.146	48.129	1.4	0.248	1.4 (70' 70'		34.423	0.655	1.48	(0' 70'	EL	6.885	0.80	0.248	1.15	(0' 70'	EL	34.423	<u> </u>
20110			33.000	•	1.468	48.444	1.4	0.248	1.89	70'		34.423	0.655	1.79	70' 70'	EL	6.885	0.80	0.248	1.47	70'		34.423	<u> </u>
	ror Er		33.075	•	1.475	48.790	1.4	0.248	1.90	70'		34.423	0.655	1.74	70'		6.885	0.80	0.248	1.48	70'		34.423	
	AIL T)		41.600	•	1.208	50.272	1.4	0.248	1.55	70'		34.423	0.655	1.58	70'		6.885	0.80	0.248	1.22	70		34.423	<u> </u>
	TR, TS,		42.000	•	1.216	51.061	1.4 1.7	0.248	1.56	70'		34.423	0.655		70'			0.80	0.248	1.22	70'		34.423	<u> </u>
	MI-L MI		42.000		1.201	52.900	1.4	0.240	1.02	70		34.423	0.655	1.44	701		6.005	0.00	0.240	1.20	70'		34.423	<u> </u>
	TRI SE		43.000	•	1 122	50 7/5	1.4	0.240	1.04	10 701		34.423	0.600	1 70	10 701		6 882		0.240	1.20	70'		34.423	
		TNAGT5R	45.000	(र)	1 117	50 088	1 <u>1</u>	0.248	1 4 7	70'	FI	34 423	0.655	1 33	'0 7∩'	FI	6 885	0.80	0.240	1 11	70'		34 423	
		FV2	28 750		1,198	57.432	1.3	0.248	2.32	70'	FI	34.423	0.655	2.08	70'	FI	6.885	0.80	0.248	1.20	70'	FI	34.423	
VEHICL	ENCY E (EV)	EV3	43.000		1.306	56.170	1.3	0.248	1.52	70'	EL	34.423	0.655	1.41	70'	EL	6.885	0.80	0.248	1.31	70'	EL	34.423	



FOR SPAN A

DESIGN ENGINEEF R.F. DeCola	OF RECORD:	DATE :	1/10/2025
DRAWN BY :	C.M. MURPHY	DATE :	<u>10/22/24</u>
CHECKED BY :	R.F. DECOLA	DATE :	10/24/24

 $\begin{pmatrix} 3 \\ 4 \end{pmatrix}$

LRFR SUMMARY

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{D\mathbf{W}}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
3 LEGAL LOAD RATING **
4 EMERGENCY VEHICLE LOAD RATING **
* * SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER- EXTERIOR RIGHT GIRDER

	PROJE	ICT NO	17	BP.IO.R	. IIO
		CABAR	RUS	CO	UNTY
	STATI	0N:	17+25.	00 -L	
	SHEET 5 C)F 5			
Robert Ducla C91B1BEB951B4FF	DEPA	stati ARTMENT	e of north car OF TRA raleigh	NSPORTA	TION
SEAL 36321	LRI 70'	FR SI CORE 120	JMMA D SL ° SK	RY F AB U Xew	OR NIT
1/10/2025	(NO	N-INTI	ERSTA	TE TRA	FFIC)
KCI Associates of N.C., P.A.	NO. BY:	REVIS	IONS	DATE:	SHEET NO. S-5
A A B C C C C C C C C C C	1		3 4		total sheets 17







DESIGN ENG R.F. DeCola	DATE :	1/10/2025	
DRAWN BY :	C.M. MURPHY	DATE :	10/22/24
CHECKED BY	: R.F. DECOLA	DATE :	10/04/24

3′-0″	PROJECT NO. <u>I7BP.IO.R.IIO</u> <u>CABARRUS</u> COUNTY STATION: <u>I7+25.00</u> -L-
	SHEET 2 OF 3 STATE OF NORTH CAROLINA
Robert Duckassesses CO1B1BEERS B44FF CARD	DEPARTMENT OF TRANSPORTATION
SEAL 36321	PLAN OF 70'-O"UNIT 27'-10"CLEAR ROADWAY 120° SKEW
1/10/2025	
NAL ETED KCI Associates of N. 4800 Falls of Neuse Road Raleigh, NC 270 Phone (919) 783-5 NC Firm License No:	No. BY: DATE: No. BY: DATE: STORE 09 1 3 509 1 509 214 2 4 4 17



CORED	SLABS	S REQ	UIRED
	NUMBER	LENGTH	TOTAL LENGTH
O'UNIT			
ERIOR C.S.	2	70'-0″	140'-0"
ERIOR C.S.	8	70'-0"	560'-0"
AL	10		700'-0"

D LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 2'-0"
70'CORED SLAB UNIT	0.6″ØL.R. STRAND
ER (SLAB ALONE IN PLACE)	2 ¹ /4″ 🕴
ECTION DUE TO RIMPOSED DEAD LOAD	3∕₄″ ↓
L CAMBER	1 ¹ ∕2″ ♦

Ε	BARRIER RAIL							
Έ	TYPE	WEIGHT						
)	STR	13'-8″	1711					
)	2	7'-2″	1181					
	LBS. 2892							
	CU.YDS. 18.1							
	LN.FT.		140.29					



BILL OF MATERIAL FOR ONE 70' CORED SLAB UNIT								
				EXTERIO	OR UNIT	INTERI	OR UNIT	
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	
B22	6	#4	STR	24'-6"	98	24'-6″	98	
S10	8	# 5	3	5'-0"	42	5'-0"	42	
S11	170	#4	3	5'-10"	662	5'-10"	662	
* S12	79	# 5	1	5'-7"	460			
S14	4	#4	4	5′-11″	16	5′-11″	16	
S15	4	# 5	3	7'-1"	30	7'-1"	30	
S16	4	#4	3	5′-11″	16	5'-11"	16	
S17	4	#4	3	6'-1"	16	6'-1"	16	
S18	4	#4	3	6′-3″	17	6′-3″	17	
REINFO	DRCING	STEEL	LBS	5.	897		897	
★ EPOX	Y COATE	ED						
REINFORCING STEEL LBS. 460								
7000 P.S.I. CONCRETE CU. YDS) a	12.0		12.0	
0.6″Ø	D.6"ØL.R. STRANDS No. 28 28							

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M2O3 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.	J
ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.	
RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.	
THE $2\frac{1}{2}^{\prime\prime}$ Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.	
THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER.SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.	
WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAS SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBM TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.	ST IT
THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE ``CONCRETE RELEASE STRENGTH'' TABLE.	
ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.	
PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.	
APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.	
GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.	
FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.	
MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.	
THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.	
FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.	
THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.	
THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STATNLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE	
THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR	7
THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN	
THE PRICE BID FOR THE PRECAST UNITS.	
EASE STRENGTH PROJECT NO. 17BP.10.R.110	_
PSI CABARRUS COUNT	Y
STATION:	_
SHEET 3 OF 3	
S Decusigned by: Refer Duchassing S L.R. 217 SEAL 36321 SEAL COPED SLAP COPED SLAP STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD 3'-O"X 2'-O" PRESTRESSED CONCRET	ΓЕ
UCURED SLAD UNIT	
KCI Associates of N.C., P.A. 4800 Falls of Neuse Road, Suite 200 Palaiata NG 27(00	NO. 8
Image: Second state of the second state of	т́s



DOCUMENT NOT CONSIDERED) FII
UNLESS ALL SIGNATURES CON	IPL



PROJECT NO. <u>178P.10.R.110</u> CABARRUS COUNTY 17+25.00 -L-STATION: _ SHEET 1 OF 6 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE END BENT 1 SHEET NO. REVISIONS S-10 NO. BY: DATE: DATE: BY: Raleigh, NC 27609 Phone (919) 783-9214 TOTAL SHEETS KC] 17 NC Firm License No: C-0764

NOTES

"U" BARS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR FOOTING DETAILS, SEE SHEET 3 OF 6. FOR WING DETAILS, SEE SHEET 4 OF 6.





	PROJE	ECT NC)17	BP.IO.R	.110
		CABAR	RUS	CC	UNTY
	STAT	ION:	17+25	.00 -L	
	SHEET 3 (DF 6			
SEAL 36321	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE END BENT				
D/2025				, , ,	
KCI Associates of N.C., P.A.	NO. BY:	REVIS		DATE:	SHEET NO. S-12
 4800 Falls of Neuse Road, Suite 200 Raleigh, NC 27609 Phone (919) 783-9214 NC Firm License No: C-0764 	1		3		total sheets 17







CHECKED BY : <u>R.F. DeCola</u>

BILL OF MATERIAL											
		ENC	<u>) BE</u>	<u>NT 1</u>				END	BE	<u>NT 2</u>	
}	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	24			<u>دا -11</u>		וט	24			<u>دا -11</u>	
	20	# 6	STR	1'-6"	45	D1	20	# 6	STR	1'-6"	45
_	10	#1	2	12'-3"	82	<u>⊔1</u>	11	#1	2	12'-3"	90
	10	#4	2	11'-10"	79	H2	11	+4	2	11'-10"	87
	22	#4	3	9'-4"	137	Н3	20	#4	3	9'-4"	125
_	16	#1	STD	3'_3"	35	K 1	16	#1	стр	31-3"	35
	10	. 4		J - J	55		10	- -	311	J - J	55
	82	# 4	STR	3'-8"	201	T1	82	#4	STR	3'-8"	201
	20	#4 #4	STR	22'-5"	299	T2 T3	20	#4 #4	STR	22'-5"	299
_	-		311	7 2	11	1.5			311	7 2	11
	41	#4	1	7'-11"	217	U1	41	#4	1	7'-11"	217
_	2	#4	1	8'-3"	11	U2	2	#4	1	8'-3"	11
	33	#4	STR	6'-4"	140	V1	28	#4	STR	6'-4"	118
	42	# 4	STR	7'-2"	201	V2	42	#4	STR	6'-2"	173
_	42	<u>#5</u> #⊿	STR	7'-2"	314	V3	42 33	#5 #⊿	STR	6'-2" 7'-0"	270 154
	20	- 4	311	1 -0	151	V 4	55	- 4	311	7 -0	134
NF	ORCI	NG_STE	EL.			REIN	FORCI	NG_STE	EL		
₹	END E	SENT 1)		2	254 LBS.	(FOR	END E	SENT 2		2	2187 LBS.
С	LASS (F	A CON OR ON	CRETE E END	BREAKDOW BENT)	N	END BE	NT 1		END	BENT 2	
Ρ	OUR *	1 FO	OTING			22.2 C.	Y.		15.9	C.Y.	
-		10 0·-				01.0.0	V		~ -	<u> </u>	
Ρ	YUUR •	OF	WINGS	WER PARI		21.8 C.	Υ.		21.5	L.Y.	
P	OUR #	1 I I P F	PER PA	RT OF		27 С	Y		27	CY	
1	UUIN	WIN				2.1 0.	1 a		2.1		
Т	OTAL	CLASS	A COI	NCRETE	2	46.7 C.	Y.		40.1	C.Y.	
					- BAR	TYP[ES -				
	2'-5″	. U1			4″			_	4″	⊨ 	
	 2'-9″	U2		$(\cdot$	$\overline{2}$						
			7				<u>}</u> - <	~		7 (3)
$\begin{array}{c c c c c c c c c c c c c c c c c c c $											
				ALL BAR	DIMENSIO	NS ARE	E OUT	TO OL	Т.		
					Pl	ROJE	СТ	NO	17	BP.IO.R	. IIO
									IC	0.0	
							CAD	ANNU	13	CO	UNIY
					S	ΤΑΤΙ	ON: _	17	+25.	<u>00 -L-</u>	-
					SHEE	T 6 01	F 6				
								STATE OF	NORTH CAF	ROLINA	
DEPARTMENT OF TRANSPORTATION											
	Rola	usigned by: H DUALA.	AR REAL	CARO				ĸ			
	C91	31BEB951B4FF	AN POF	ESSION			SI	JBST	RUC	TURE	
				EAL				REN	IΤ	N∩ 1-	-2
			د : ۸. ل	0321	1999 999		NU			тя с. 1 С	۲
ULIALLS											
	1	/10/2025	" 0 2 M /	F. DEVISION							
			KC	CI Associates of N.C	., P.A.	BY.	F	REVISIONS			SHEET NO. S-15
N	AL.		4800 F	Calls of Neuse Road, Raleigh, NC 2760	Suite 200	ויט. 	UAIE:	<u>∾∪.</u> 3	01:	DAILS	TOTAL
ET	'ED	I KU		- 1010 (919) 783-92 Firm License No. (<u> </u>			A			17

	PROJECT NO. <u>17BP.10.R.110</u>
— GROUND LINE	STATION: 17+25.00 -L-
DocuSigned by: Robust Ducla C91B1BEB951B4FF 1/10/2025	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD RIP RAP DETAILS
KCLAssociates of N.C. P.A.	REVISIONS SHEET NO.
IALIEDImage: A constraint of the	NO. BY: DATE: NO. BY: DATE: S-16 1 3 3 TOTAL SHEETS 17 TOTAL SHEETS 17

ESTIMATED QUANTITIES					
E AT /+25.00 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE			
	TONS	SQUARE YARDS			
BENT 1	115	130			
BENT 2	85	95			

DRAWN BY :

CHECKED BY : R.F. DECOLA

___ DATE : <u>10/24/24</u>

DESIGN DATA:

SPECIFICATIONS	AASHTO (CURRENT)
LIVE LOAD	- SEE PLANS
IMPACT ALLOWANCE	- SEE AASHTO
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50W _	27,000 LBS. PER SQ. IN.
- AASHTO M270 GRADE 50	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR	SEE AASHTO
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS. PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS. PER CU. FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO $1\frac{1}{2}$ " RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A $\frac{1}{4}$ " FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ " RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " \emptyset SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{4}$ " \oslash STUDS FOR 4 - $\frac{3}{4}$ " \oslash STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " \oslash STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " \emptyset STUDS BASED ON THE RATIO OF 3 - $\frac{5}{4}$ " \emptyset STUDS FOR 4 - $\frac{3}{4}$ " \oslash STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{6}$ " IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EOUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES. THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY $\frac{1}{16}$ " OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY. IN CASE OF DISCREPANCY. THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.